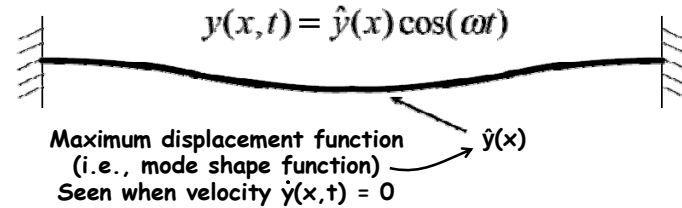


Lecture 20: Resonance Frequency & Equiv Ckts. I

- Announcements:
- Project handed out and described last time
- You should be partnered up, 3 per team
- -----
- Reading: Senturia, Chpt. 10: §10.5, Chpt. 19
- Lecture Topics:
 - ↳ Estimating Resonance Frequency
 - ↳ Lumped Mass-Spring Approximation
 - ↳ ADXL-50 Resonance Frequency
 - ↳ Distributed Mass & Stiffness
 - ↳ Folded-Beam Resonator
 - ↳ Resonance Frequency Via Differential Equations
- Reading: Senturia, Chpt. 5
- Lecture Topics:
 - ↳ Lumped Mechanical Equivalent Circuits
 - ↳ Electromechanical Analogies
- -----
- Last Time:
- Started resonance frequency determination

↻ over

- Vibrating structure displacement function:

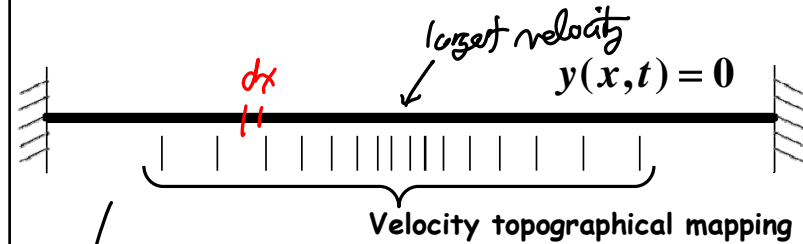


- Procedure for determining resonance frequency:

- ↳ Use the static displacement of the structure as a trial function and find the strain energy W_{max} at the point of maximum displacement (e.g., when $t=0, \pi/\omega, \dots$)
- ↳ Determine the maximum kinetic energy when the beam is at zero displacement (e.g., when it experiences its maximum velocity)
- ↳ Equate energies and solve for frequency

Get Maximum Kinetic Energy

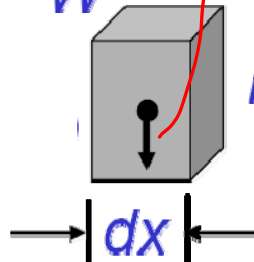
velocity: $v(x) = \frac{\partial y(x,t)}{\partial t} = -\omega \hat{y}(x) \sin \omega t$



When $y(x,t) = 0$, all the energy in the structure is kinetic ($W = 0$)

$v(x, \frac{(2m+1)\pi}{2\omega}) = -\omega \hat{y}(x)$

$t = \frac{\pi}{2\omega}, \frac{3\pi}{2\omega}, \dots$



velocity $v = -\omega \hat{y}(x)$ $\frac{(2m+1)\pi}{2w}$

$$dK = \frac{1}{2} \cdot dm \cdot [v(x,t)]^2$$

$$dm = \rho(W h dx)$$

density

Maximum K.E.:

$$K_{max} = \int_0^L \frac{1}{2} \rho W h dx v^2(x,t) = \int_0^L \frac{1}{2} \rho W h \omega^2 [\hat{y}(x)]^2 dx$$

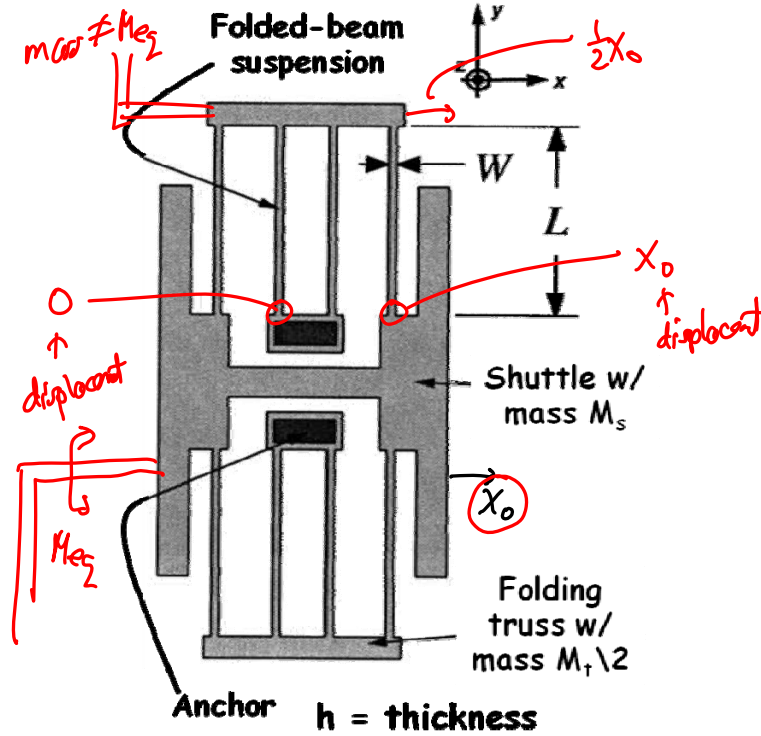
To get frequency:

$$K_{max} = W_{max}$$

$$\therefore \omega = \sqrt{\frac{W_{max}}{\int_0^L \frac{1}{2} \rho W h [\hat{y}(x)]^2 dx}}$$

ω : radian resonance freq. W : beam width
 W_{max} : max. potential energy h : " thickness
 ρ : density of the structural material
 $\hat{y}(x)$: resonance mode shape

Resonance freq. of a Folded-Beam Structure



Folded-beam suspension

Shuttle w/ mass M_s

Folding truss w/ mass $M_t/2$

Anchor $h = \text{thickness}$

X_0 displacement

$\frac{1}{2} X_0$ displacement

$M_{t/2}$

- Derive an expression for the resonance frequency of the above structure

Use the Rayleigh-Ritz Method: (energy method)

$$KE_{max} = PE_{max}$$

Find the kinetic energy \rightarrow one piece at a time

$$KE_{max} = \underbrace{KE_s}_{\text{shuttle}} + \underbrace{KE_t}_{\text{truss}} + \underbrace{KE_b}_{\text{beams}}$$

$$KE_{max} = \frac{1}{2} v_s^2 M_s + \frac{1}{2} v_t^2 M_t + \frac{1}{2} \int v_b^2 dM_b$$

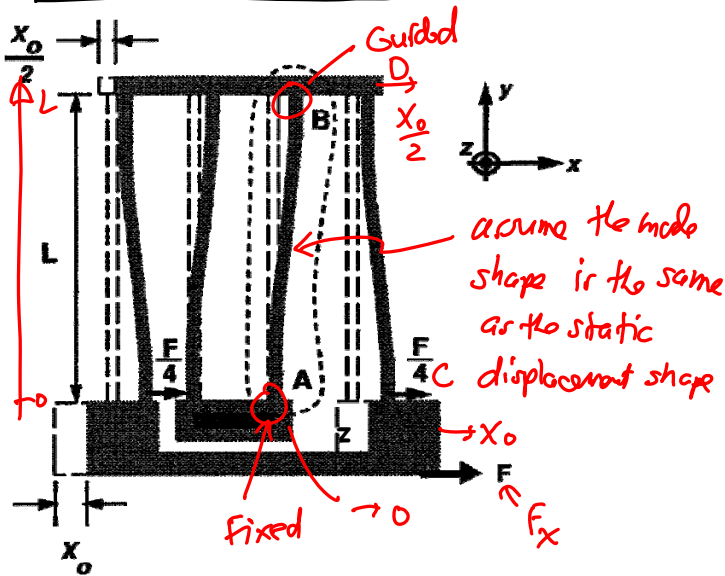
Velocity of the Shuttle: $v_s = \omega_0 x_0$

$\rho V =$ static mass
 \downarrow
 $\therefore KE_s = \frac{1}{2} v_s^2 M_s = \frac{1}{2} \omega_0^2 x_0^2 M_s$
 resonance freq. \uparrow maximum displacement of the shuttle

Velocity of Truss: $v_t = \frac{1}{2} v_s = \frac{1}{2} \omega_0 x_0$

$\therefore KE_t = \frac{1}{2} \left(\frac{1}{2} \omega_0 x_0\right)^2 M_t = \frac{1}{8} \omega_0^2 x_0^2 M_t = KE_t$
 static mass for both trusses

Velocity of the Beam Segments:



Segment [AB]:

$$\tilde{x}(y) = \frac{F_x}{48EI_z} (3Ly^2 - 2y^3), \quad 0 \leq y \leq L \quad (1)$$

At $y=L$: $x(L) = \frac{x_0}{2} = \frac{F_x L^3}{48EI_z} \leftarrow$ B.C.

Substitute into (1):

$$\tilde{x}(y) = \frac{x_0}{2} \left[3\left(\frac{y}{L}\right)^2 - 2\left(\frac{y}{L}\right)^3 \right]$$

which yields for velocity:

$$v_b(y)_{[AB]} = \frac{x_0}{2} \left[3\left(\frac{y}{L}\right)^2 - 2\left(\frac{y}{L}\right)^3 \right] \omega_0$$

Plugging into the expression for KE_b :

$$KE_{[AB]} = \frac{1}{2} \int_0^L \frac{x_0^2 \omega_0^2}{4} \left[3\left(\frac{y}{L}\right)^2 - 2\left(\frac{y}{L}\right)^3 \right]^2 dM_{[AB]}$$

$$= \frac{x_0^2 \omega_0^2}{8L} M_{[AB]} \int_0^L \left[3\left(\frac{y}{L}\right)^2 - 2\left(\frac{y}{L}\right)^3 \right]^2 dy$$

$M_{[AB]}$ is the static beam mass per unit length

$$KE_{[AB]} = \frac{13}{280} x_0^2 \omega_0^2 M_{[AB]}$$

mass modification factor

For segment [CD]:

$$v_y(y)|_{[CD]} = X_0 \left[1 - \frac{3}{2} \left(\frac{y}{L} \right)^2 + \left(\frac{y}{L} \right)^3 \right] \omega_0$$

Thus:

$$KE_{[CD]} = \frac{X_0^2 \omega_0^2 M_{[CD]}}{2L} \int_0^L \left[1 - \frac{3}{2} \left(\frac{y}{L} \right)^2 + \left(\frac{y}{L} \right)^3 \right]^2 dy$$

$$KE_{[CD]} = \frac{83}{280} X_0^2 \omega_0^2 M_{[CD]}$$

static mass of beam [CD]

Let $M_b \triangleq$ total mass of all 8 beams

Then: $M_{[AB]} = M_{[CD]} = \frac{1}{8} M_b$

Thus:

$$KE_b = 4KE_{[AB]} + 4KE_{[CD]} = \frac{6}{35} X_0^2 \omega_0^2 M_b$$

and

$$KE_{max} = X_0^2 \omega_0^2 \left[\frac{1}{2} M_s + \frac{1}{8} M_t + \frac{6}{35} M_b \right]$$

$PE_{max} \rightarrow$ simply equal to work done to achieve maximum deflection (X_0)

$$PE_{max} = \frac{1}{2} k_x X_0^2$$

$k_c \leftarrow$ stiffness of a cantilever of length $\frac{1}{2}L$

Thus, using Rayleigh-Ritz:

$$KE_{max} = PE_{max}$$

$$X_0^2 \omega_0^2 \left[\frac{1}{2} M_s + \frac{1}{8} M_t + \frac{6}{35} M_b \right] = \frac{1}{2} k_x X_0^2$$

$$\omega_0 = \left[\frac{k_x}{M_{eq}} \right]^{1/2} = k_c$$

\uparrow
where $M_{eq} = M_s + \frac{1}{4} M_t + \frac{12}{35} M_b$

(Resonance Freq. of a Folded-Beam Suspended Shuttle)

- Looked briefly at Module 10 slides 21-31, but very quickly - you should go through it again on your own

Equivalent Dynamic Mass

Location on Folding - *

Truss $\rightarrow M_{eq}(truss)$

Location on Shuttle: $M_{eq}(shuttle)$

$M_{eq} = \infty$

Equivalent Mass:

Equiv. Mass: $M_{eq,x} = \frac{KE_{max}}{\frac{1}{2}V_x^2} = \frac{KE_{max}}{\frac{1}{2}V_x^2}$

for beam $\downarrow \frac{1}{2}PA \int_0^L V^2(x) dx$

velocity @ location x

$M_{eq}(shuttle) = \frac{KE_{max}}{\frac{1}{2}V_{shuttle}^2} = \frac{KE_{max}}{\frac{1}{2}V_{shuttle}^2}$

$M_{eq}(shuttle) = M_s + \frac{1}{4}M_t + \frac{12}{35}M_b$

static masses: $\rho(\text{volume})$

* $\rightarrow \frac{\omega_0^2 x_0^2 (\frac{1}{2}) [M_s + \frac{1}{4}M_t + \frac{12}{35}M_b]}{\frac{1}{2} (\frac{1}{4}) \omega_0^2 x_0^2}$

$M_{eq}(truss) = 4 [M_s + \frac{1}{4}M_t + \frac{12}{35}M_b]$

equiv. dynamic mass @ the truss location

Equiv. Dynamic Stiffness

$\omega_0 = \sqrt{\frac{K_{eq}(x)}{M_{eq}(x)}} \rightarrow K_{eq}(x) = \omega_0^2 M_{eq}(x)$

\Rightarrow large equiv. mass & large stiffness go hand-in-hand

Equiv. Dynamic Damping

$Q = \frac{\omega_0 M_{eq}(x) \sim L}{C_{eq}(x) \sim R} \rightarrow C_{eq}(x) = \frac{\omega_0 M_{eq}(x)}{Q} = \frac{K_{eq}(x) M_{eq}(x)}{Q}$

damping

specified @ single location, x