

**Lecture 13: Beam Combos II & Energy Methods**

- Announcements:
- HW#3 due tomorrow morning
- HW#4 online soon; due next Wednesday morning
- Module 9 on Energy Methods is online
- Midterm is next week, Thursday, March 19
- Prof. Nguyen's Wednesday office hours cancelled this week (since I'm giving a tutorial on MEMS-Based Oscillators for BSAC Industry at this time)

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- Reading: Senturia, Chpt. 9

- Lecture Topics:

- ↳ Bending of beams
- ↳ Cantilever beam under small deflections
- ↳ Cantilever with residual stress gradient
- ↳ Combining cantilevers in series and parallel
- ↳ Folded suspensions
- ↳ Design implications of residual stress and stress gradients for folded-beam devices

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- Reading: Senturia, Chpt. 10

- Lecture Topics:

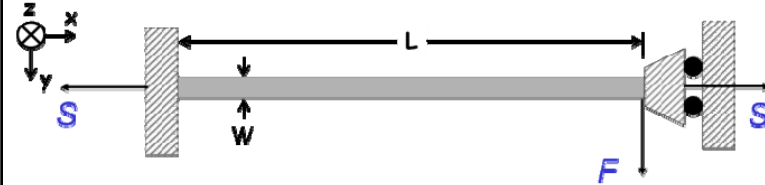
- ↳ Energy Methods
- ↳ Virtual Work
- ↳ Energy Formulations
- ↳ Tapered Beam Example
- ↳ Estimating Resonance Frequency

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- Last Time:

Tensioned Spring (Non-Ideality)

- Important case for MEMS suspensions, since the thin films comprising them are often under residual stress
- Consider small deflection case:  $y(x) \ll L$



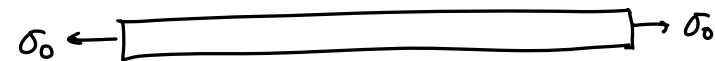
Governing differential equation: (Euler Beam Equation)

$$EI \frac{d^4 y}{dx^4} - S \frac{d^2 y}{dx^2} = F \delta(x-L)$$

Axial Load
Unit impulse @  $x=L$

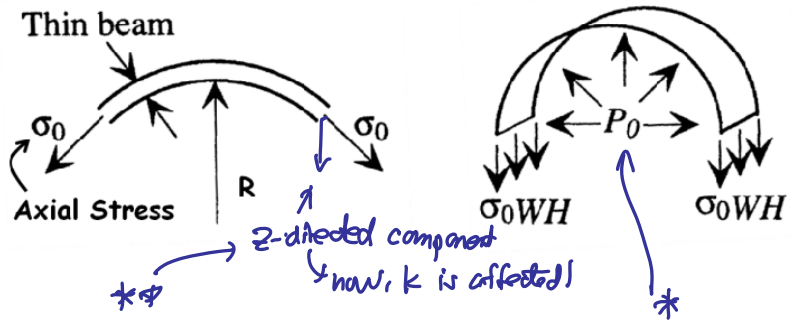
Heuristic Derivation for the Euler Beam Equation

Consider first a straight beam under axial stress:



⇒ no effect on z-directed stiffness when the beam is straight!

...but when the beam is bent:

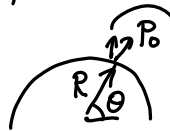


\* Upward pressure  $P_0$  to counteract the downward force from  $\dots$  to keep everything in static equilibrium

For ease of analysis:

Assume the beam is bent to an angle  $\pi$   
 downward normal force:  $2\sigma_0 W H$

Upward force due to  $P_0$ :



$$P_y(\theta) = P_0 \sin \theta$$

$$F_u = \int_0^\pi (P_0 \sin \theta) W(R d\theta)$$

$$= -P_0 W R \cos \theta \Big|_0^\pi$$

$$= 2RW P_0$$

[Equilibrium]  $\Rightarrow 2RW P_0 = 2\sigma_0 W H \rightarrow P_0 = \frac{\sigma_0 H}{R}$

$\left[ q_0 = \frac{\text{beam load}}{\text{unit length}} = P_0 W, \frac{1}{R} = \frac{d^2 w}{dx^2} \right] \rightarrow$  2-directed beam displ.

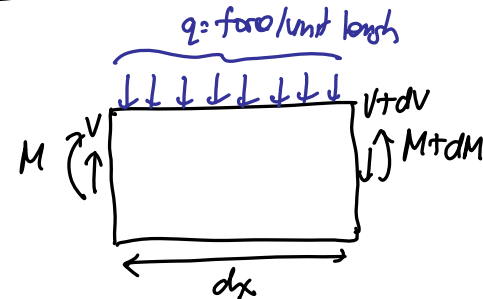
$q_0 = \sigma_0 W H \frac{d^2 w}{dx^2}$  generalizes to the case of smaller displacements & angles

Using the differential beam bending Equation

$$\frac{d^2 w}{dx^2} = -\frac{M}{EI} \rightarrow \frac{d^4 w}{dx^4} = \frac{q}{EI}$$

load / unit length

Relationships Between Forces on a Fully Loaded Differential Beam Element



[Total Static Equilibrium]  $\Rightarrow$  total force = 0

$$F_T = \text{total force} = q dx + (V+dV) - V = 0$$

$$\therefore \frac{dV}{dx} = -q \quad (1)$$

$\Rightarrow$  also, total moment w/r to the left hand edge = 0

$$M_T = (M+dM) - M - (V+dV) dx - \frac{1}{2} q dx^2 = 0$$

[neglect products of differentials]  $\int_0^{dx} (q du) u = \frac{1}{2} q dx^2$

$$dM - V dx = 0 \rightarrow \frac{dM}{dx} = V \quad (2)$$

Using (1) & (2):

$$\frac{d^2 M}{dx^2} = \frac{dV}{dx} = -q$$

external load

$$EI \frac{d^4 w}{dx^4} = q + q_0 \leftarrow \text{equiv. load f/ axial stress}$$

$[q_0 = \sigma_0 W H \frac{d^2 w}{dx^2}]$

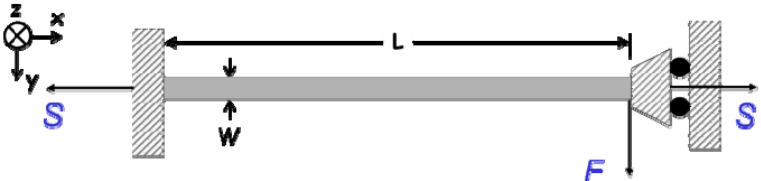
$$EI \frac{d^4 w}{dx^4} - (\sigma_0 W H) \frac{d^2 w}{dx^2} = q$$

tension in the beam =  $S$   
 a force

Euler Beam Equation

**Clamped-Guided Beam Under Axial Load**

- Important case for MEMS suspensions, since the thin films comprising them are often under residual stress
- Consider small deflection case:  $y(x) \ll L$



Governing differential equation: (Euler Beam Equation)

$$EI \frac{d^4 y}{dx^4} - S \frac{d^2 y}{dx^2} = F \delta(x-L)$$

Axial Load      Unit impulse @  $x=L$

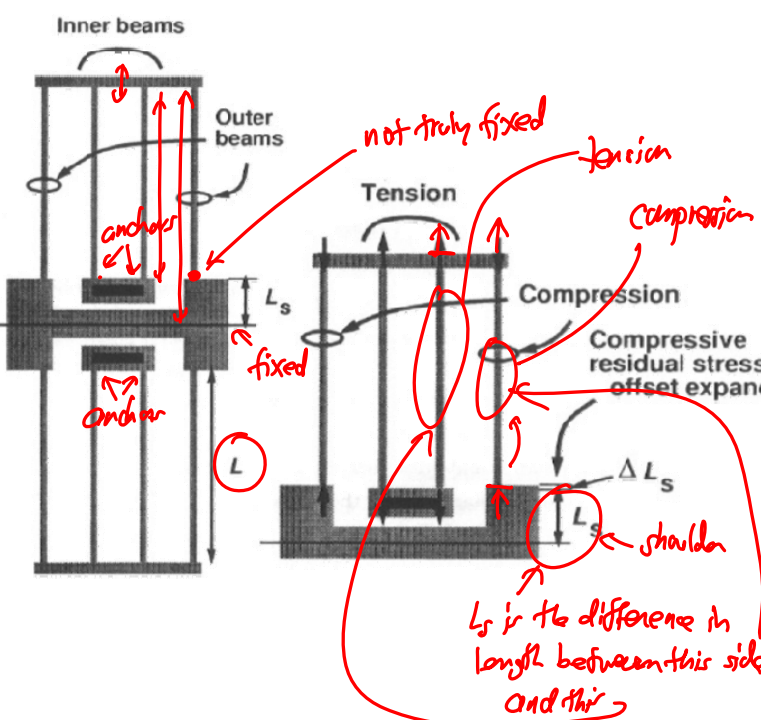
- Can solve the ODE using standard methods
- Senturia, pp. 232-235: solves ODE for case of point load on a clamped-clamped beam (which defines B.C.'s)
- For solution to the clamped-guided case: see S. Timoshenko, Strength of Materials II: Advanced Theory and Problems, McGraw-Hill, New York, 3rd Ed., 1955
- Result from Timoshenko:

$$S > 0 \text{ (tension)} \quad k^{-1} = \frac{pL - 2 \tanh(pL/2)}{p|S|} = \frac{y(x=L)}{F}$$

$$S < 0 \text{ (compression)} \quad k^{-1} = \frac{-pL + 2 \tanh(pL/2)}{p|S|} = \frac{y(x=L)}{F}$$

where  $p = \sqrt{\frac{|S|}{EI}}$

force



Inner beams

Outer beams

anchors

fixed

shoulder

Tension

Compression

Compressive residual stress: offset expands

$L$

$L_s$

$\Delta L_s$

$L_s$  is the difference in length between this side and this

① If polysil strain is  $\epsilon_r$ , then substrate expands by  $\Delta L_s = \epsilon_r L_s$

② This then applies a load to the beams,  $\Delta L = \Delta L_s$

③ Beam strain & stress

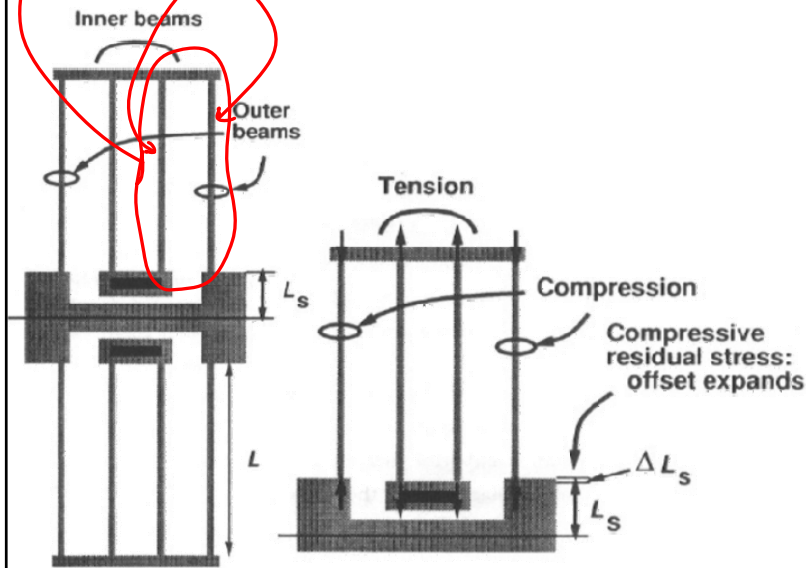
$$\epsilon_b = \frac{\Delta L}{2L} + \frac{\Delta L_s}{2L} = \pm \epsilon_r \frac{L_s}{2L}$$

Shear force:  $S = \pm E \epsilon_r \left(\frac{L_s}{2L}\right) Wh$  (axial tension)

④ Spring Constant:

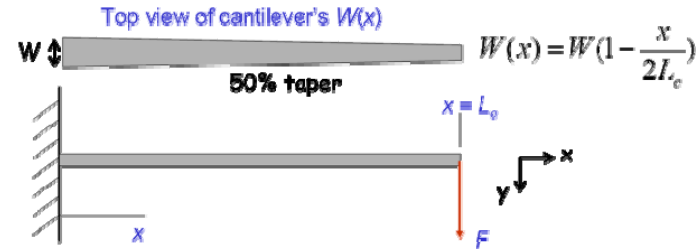
$$k = 4(k_{com}^{-1} + k_{ten}^{-1})^{-1}$$

$$k = 4 \left[ \frac{-pL + 2 \tan(pL/2)}{p|S|} + \frac{pL - 2 \tanh(pL/2)}{p|S|} \right]^{-1}$$

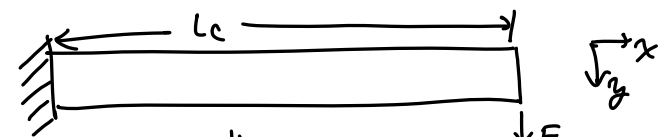


More General Geometries

- Euler-Bernoulli beam theory works well for simple geometries
- But how can we handle more complicated ones?
- **Example:** tapered cantilever beam
- **Objective:** Find an expression for displacement as a function of location  $x$  under a point load  $F$  applied at the tip of the free end of a cantilever with tapered width  $W(x)$



Same Problem As Before: Take a beam, apply a force.



- ① Apply force.
- ② Beam responds by bending
- ③ This force has done work:  $W = F \cdot \eta(L_c)$
- ④ Strain generated  
 ↳ so the beam has received an influx of stored energy  
 ↳ magnitude of " " determined by the shape the beam takes!

⑤ Then

$$U = \text{Stored Energy} - \text{Work Done} \rightarrow 0$$

$y(x) = f(x)$  } When we choose the right shape!  
 ↓  
 This is how we get the beam's response to force F.

**Fundamentals of Energy Density**

General Definition of Work:

$$W(q) = \int_0^q e(q) dq \quad \begin{matrix} q = \text{displacement} \\ e = \text{effort} \end{matrix}$$

for EE:  $W(Q) = \int_0^Q \frac{Q}{C} dQ$

**Strain Energy Density**

$$W = \int_0^{\epsilon_x} \sigma_x d\epsilon_x$$

value of strain @ position (x,y,z)

$\sigma_x = f(\epsilon_x)$  → relates stress to strain @ position (x,y,z)

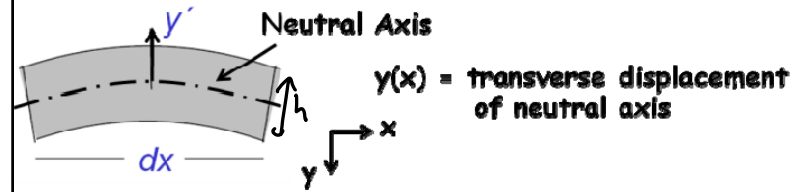
$[\sigma_x = E\epsilon_x]$  ↓  
 $W = \int_0^{\epsilon_x} E\epsilon_x d\epsilon_x = \frac{1}{2} E\epsilon_x^2$

Total Strain Energy: [J]

volume ↓

$$W = \iiint \left( \frac{1}{2} E (\epsilon_x^2 + \epsilon_y^2 + \epsilon_z^2) + \frac{1}{2} G (\gamma_{xy}^2 + \gamma_{xz}^2 + \gamma_{yz}^2) \right) dV$$

**Bending Energy Density**



First, find the bending energy  $dW_{\text{bend}}$  in an infinitesimal length  $dx$ :

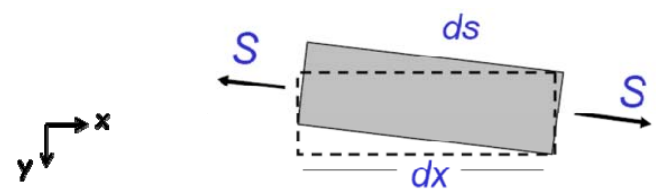
$$dW_{\text{bend}} = W dx \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{1}{2} E \epsilon_x^2(y') dy'$$

$\left[ \frac{1}{R} = \frac{d^2y}{dx^2}, \epsilon_x = \frac{y'}{R} \right] \Rightarrow \epsilon_x(y') = y' \frac{d^2y}{dx^2}$

$$dW_{\text{bend}} = W dx \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{1}{2} E \left[ y' \frac{d^2y}{dx^2} \right]^2 dy' = \frac{1}{2} E \underbrace{\left( \frac{Wh^3}{12} \right)}_{I_2} \left( \frac{d^2y}{dx^2} \right)^2 dx$$

∴  $W_{\text{bend}} = \frac{1}{2} E I_2 \int_0^L \left( \frac{d^2y}{dx^2} \right)^2 dx$

Energy Due to Axial Load



⇒ energy due to lengthening:

$$ds = [(dx)^2 + (dy)^2]^{1/2} = dx \left[ 1 + \left( \frac{dy}{dx} \right)^2 \right]^{1/2}$$

[binomial theorem]  $\rightarrow \approx dx \left[ 1 + \frac{1}{2} \left( \frac{dy}{dx} \right)^2 \right]$

$$\therefore \epsilon_x = \frac{ds - dx}{dx} = \frac{1}{2} \left( \frac{dy}{dx} \right)^2$$

$$\left[ dW_{axial} = S \epsilon_x dx = \frac{1}{2} S \left( \frac{dy}{dx} \right)^2 dx \right]$$

$$W_{axial} = \frac{1}{2} S \int_0^L \left( \frac{dy}{dx} \right)^2 dx$$

↑  
Axial Strain Energy

- Go to Module 9, pg. 10, and look at shear strain energy
- Then, finish off Module 9